

Experimental Studies on Various Different Section of Cold Formed Steel Structure in Beam and Column

Saurabh Kumar Singh¹ and Madan Chandra Maurya²

¹M.Tech. Structure, Madan Mohan Malviya University of Technology

²Madan Mohan Malviya University of Technology

Abstract—Cold-formed steel (CFS) made by rolling or pressing steel into semi-finished or finished goods at relatively low temperatures i.e. room temperature. CFS members have been used in roof or truss member of buildings, racks, aircraft bodies, car bodies, railway coaches, transmission towers & poles etc. CFS has highest strength-to-weight ratio, layer of zinc and other metallic coating steel provides long-term durability, it is recyclable, cost savings on a number of fronts, being light in weight it is easily transportable. The main aim is to analyze the various section of CFS with correspondence to specific hot rolled section and compare their various properties like strength-to-weight ratio. It can be done by changing the orientation of element in CFS and increasing its moment of inertia value by increasing the area on flange part. The hot-rolled steel members have been recognized as the most popularly and widely used steel group. But because of its several advantages over the hot-rolled steel sections, the use of cold-formed high strength steel structural members has rapidly increased lately. However, the structural behavior of this light gauge steel members characterized by various buckling modes such as local buckling, distortional buckling, flexural torsional buckling. It is therefore important that this buckling modes should be delayed or eliminated completely in order to increase the ultimate load carrying capacity of cold-formed steel members. Open cold formed steel sections such as C, Z, hat sections are commonly used because of their simple forming and easy connections, but they suffer from certain buckling modes due to their mono symmetric or point symmetric nature, high plate slenderness, eccentricity of shear center to centroid and low torsional rigidity. Being several advantages over hot rolled steel section up till now it is used as beam for supporting roof truss and other light weight element it is not used as main element of structure like Beam, Column or slab. Hence there is lot of scope for future research in this area.

1. INTRODUCTION

Cold formed steel are also called light gauge steel section which are formed by steel sheet. Cold-formed sections are produced by rolling and punching flat sheet steel at room temperatures. The thickness of sheet used in CFS is usually 1 to 3 mm. Thicker material up to 12 mm are formed if pre-galvanized material are not required for the particular application. Normally, the yield strength of steel sheets used to make CFS is 280 N/mm², although there is a trend to use steels of higher strengths, and as low as 230 N/mm². Manufacturing of CFS require steel coils of 1.0 to 1.25 m width, lay them

longitudinally to the appropriate width of required section and feed them into a series of roll forms. These rolls, containing male and female dies, which are arranged in pair and move in opposite direction so that as the sheet is feed through them and shape is gradually altered to the required profile. The number of pairs of rolls depends on the complexity of the cross section shapes. At the end of the rolling stages a flying shearing machine cuts the member into desired shape. Cold-Form Steel buildings section are a predetermined assembly of structural element that has proven over time to meet a wide range of structural and aesthetic requirements. Cold-Form Steel Structure concept originated during World War II in 1960's in the United States and made available in India in late 90's. During World War II, it is known as Pre-fabricated building. Later on which became a household need and was mass produced by hundreds of thousands to meet a requirement for inexpensive and standardized shelter. Which require no special skills, these structures are assembled with only hand tools and with no greater effort could be readily dismantled and moved and re-erected somewhere else. The scientific term Cold-Form Steel buildings came into being in the 1960's. The buildings were "Cold-Form Steel" because like their ancestors, they relied upon standard engineering designs for a limited number of off the shelf configurations. As long as the purchaser standard designs the buildings could be properly called Cold-Form Steel.

Cold forming increases the yield strength of steel, it enhance the mean yields stress by 15%-30%. The increase being the consequence of cold working well into the strain-hardening range. These increases are predominant in zones where the metal is bent by folding. For purpose of design, the yield stress may be regarded as having been enhanced by a minimum of 15%. Some of the main advantages of CFS as compared with their hot-rolled counterparts are as follows:

- CFS has high strength to weight ratio.
- Being light in weight it is easily transportable.
- Pre-galvanized or pre-coating metals are formed, so that high resistance to corrosion can be achieved.

- CFS can be employed to produce any desirable shape and size.

In 1984 C.C Weng (Compression test on cold form steel section)

He uses 93 columns to check compressive strength out of which 68 are long and 25 are short column. He finds out the flexural buckling strength of section. The stress and strain relationship and forming all operations while checking the proportional limits and comparing the theoretical values with experimental values.

In 2004 Juile Mill (self-drillings screw joint for cold form steel channel portal) he studied about the joints used in CFS and done testing on them. Since U.S. sections having a lower design capacity than the Australian section. However this does not mean that the same problems will occur with the conventional joints and hence he proposed self-drilling screw joints which are a valid option to overcome this problem in both countries.

In 2008 Panagiots Frantzis (Durability of cold form steel) Since joints are subjected to various loads in the presence of room temperature. A graphite gauge technique is developed which is used to monitor the incubation for the time of a crack to form and measure its subsequent velocity as caused of applied failure energy. Two methods are used first is durability testing ring and second is side and end projection formation was brittle in nature compared to the model not exceed 40 J/m².

In 2009 Gillbert H. Begain (Light gauge cold formed steel profile) for decks in housing unit's sheet metal can provide various components with the CFS to meet the various need of the construction of components at fewer prices. A new types of channel profiles has been developed and applied for building the deck of a family house this channel profile space for heating ventilating and electrical conducts designing innovative steel light gauge component thought ingenuity of the designer. the light weight bring ducts. as regards acoustics but it reduces the load on the foundation.

Concluding Remark on Literature Work: With the different studies based on the beams and columns of different CFS section increase in load carrying capacity, deflection capacity and stiffness as compare to respective hot rolled steel section.

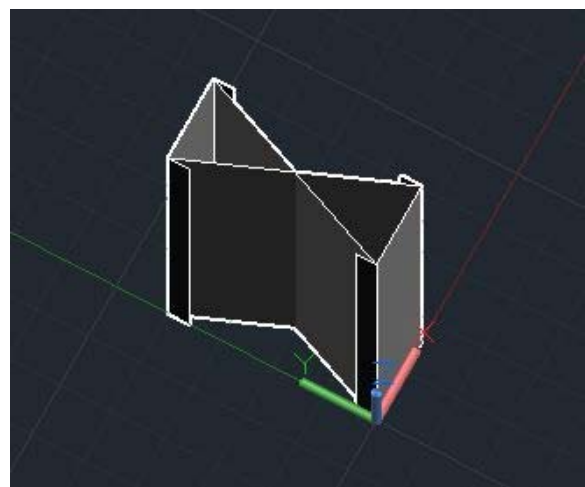
2. RESEARCH METHODOLOGY

A forging section where punching or pressing is done comprises of die block which have two component male and female over which work piece is positioned in such a way that desired shape is obtained. Then steel sheet is feed in to die and desired shape is obtained. While forging bending is come over both the side i.e. compressive and tensile side both these forces resist the bending so the bending angle has to keep increases as compare to desired bending angle as if this is not

done the residual stress which is left in material tries or let the material to regain its original shape. While bending sheet metal length is stretches. The bend deduction is the amount the sheet metal will stretch when bent as measured from the outside edges of the bend. The bend radius refers to the inside radius. So this lead to conclusion that bending of CFS depend on material type, property and bending angle thus the bending radius depend on above conclusion.

Evaluation on strain-gauge measurements (critical load determination and post-critical behavior) for deflection angle and on factors influencing the load carrying capacity (web buckling and load application). CFS sheets of rectangular section are widely used for buildings in India.

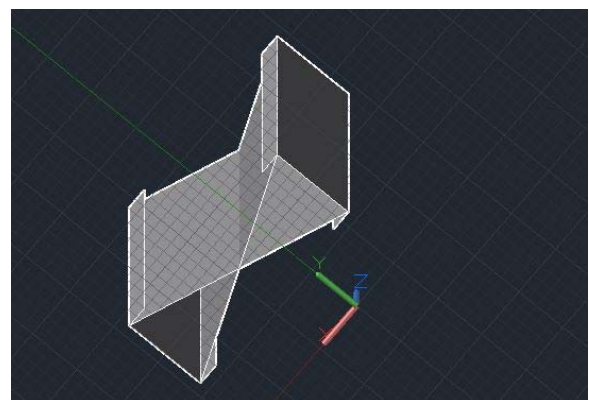
In our experiment we will take section as shown below



For column section in which lip width is 11.8mm, base and top width is 138.8mm thickness of plate used as 3mm and various other properties are as under (approximate value):-

Area	1240 mm ²
I _{xx}	7.89*10 ⁶ mm ⁴
I _{yy}	2.36*10 ⁶ mm ⁴
Z _{yy}	23.56*10 ³ mm ³

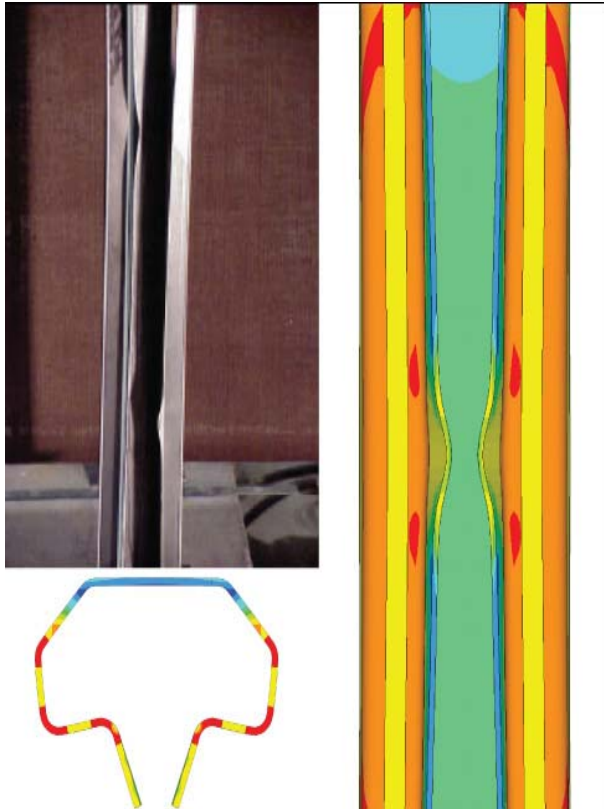
For Beam section:-



For column section in which lip width is 11.8mm, base and top width is 138.8mm thickness of plate used as 3mm and various other properties are as under(approximate value):-

Area	1240 mm ²
I _{yy}	7.89*10 ⁶ mm ⁴
Z _{yy}	89.4*10 ³ mm ³
I _{xx}	2.36*10 ⁶ mm ⁴
Z _{xx}	23.56*10 ³ mm ³

In these two above section when loading is done buckling is shown as



Shows necking in columns when the load is applied for checking flexure strength of columns.

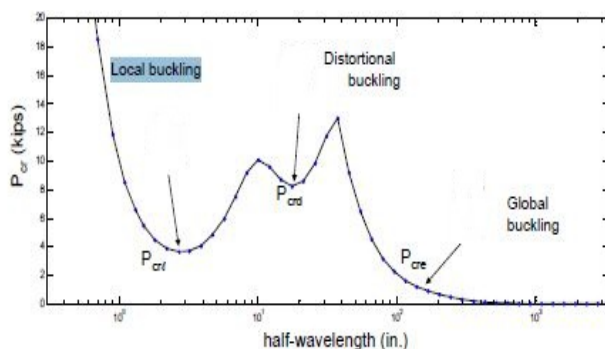


Figure shows Column elastic buckling curve generated with CUFSM

3. CONCLUSION

Open cold formed steel sections such as C,Z ,hat sections are commonly used because of their simple forming and easy connections, but they suffer from certain buckling modes due to their mono symmetric or point symmetric nature, high plate slenderness, eccentricity of shear center to centroid and low torsional rigidity. Being several advantages over hot rolled steel section up till now it is used as beam for supporting roof truss and other light weight element it is not used as main element of structure like Beam, Column or slab. Hence there is lot of scope for future research in this area.\

REFERENCES:

- [1] AISI Stanard, "North American specification for design of steel members", July 2007.
- [2] B.W. Schafer and S. Adany (2006) "Buckling analysis of cold-formed steel members using CUFSM", 18th International Conference on CFS, October 26- 27,2006.
- [3] B.W. Schafer "Designing Cold-Formed Steel Using the Direct Strength Method", 18th International Specialty Conference on Cold-Formed Steel Structures October 26-27, 2006.
- [4] CilmarBasaglia and Dinar Camotim "Buckling, Postbuckling, Strength, and DSM Design of Cold-Formed Steel Continuous Lipped Channel Beams", Journal of structural Engineering.2013.139: 657,668.
- [5] Dr. B.C. Punmia, Ashok Kumar Jain, Arun Kumar. Jain, "Design of Steel Structures", Jan 1998,pp 561-600.
- [6] Indian Standard IS: 801-1975, "Code of practice for use of cold-formed light gauge steel structural members in general building construction", Bureau of Indian Standards, New Delhi (1976).
- [7] L. C. M. Vieira, B. W. Schafer, "Behavior and Design of Sheathed Cold-Formed Steel Stud Walls under Compression" J. Struct. Eng. 2013.139:772-786.
- [8] R.B. Kulkarni, ShwetaB.Khidrapure, "Parametric study and comparison of Indian standard code with British code for the Design of Light gauge cold formed flexural members", International Journal of Engineering and Technical Research (IJETR) ISSN: 2321-0869, Volume-2, Issue-11, November 2014.
- [9] Somadasa Wanniarachchi, "Flexural behavior and Design of Cold formed steel beams with rectangular hollow flanges", December 2006.Vijayasimhan M , Marimuthu V , Palani G.S and Rama Mohan Rao P (2013) "Comparative Study on Distortional Buckling Strength of Cold-Formed Steel Lipped Channel Sections", Research Journal of Engineering Sciences,Vol 2(4), 10-15 April (2013).
- [10] B.plim (1987). "Distortional buckling formulas forChannel columns." J. Struct. Engrg. ASCE, 133(5), 1063-1078.
- [11] Béguin, G. H. 2009. "A graphical determination of the shear center in thin-walled asymmetrical U-profiles." Pract.Period.Struct.Des.Constr., accepted for publication.
- [12] Béguin, G. H., and Trudel, F. _1975_. "IstdieAnwendung von StahlblecimWohnungs- und Hochbaugerechtfertigt?" BlechRohreProfile,Prost&MeinerVerlag, Coburg, Germany, 216-221.
- [13] Cold Form Steel Code:IS 801

- [14] Desmond, T. P. (1978). "The behavior and strength of thin-walled compression elements with longitudinal stiffeners." Research Report No. 369, Dept. of Struct.
- [15] Duncan, B., and Crocker, L. 2001 Characterization of flexible adhesives for design, NPL Publications, Teddington, Middlesex, U.K.
- [16] Dunn, D. J. 2004 "Engineering and structural adhesives." Rapra Review Reports, FLD Enterprises Inc., Rapra Limited, Shropshire, U.K., 15(1), Rep. No. 169.
- [17] Engrg., Cornell Univ., Ithaca, N.Y.
- [18] Evans, A. G. 1972 "A method for evaluating the time dependence failure characteristics of brittle materials and its application to polycrystalline alumina." *J. Mater. Sci.*, 7, 1137–1146
- [19] George winter" sustanbilty of cold form steel" DOI:10.1061/ASCE ISSN 0882-5628/1245/663/20-234 dec2012
- [20] J. Whittle and C. Ramseyer, "Buckling capacities of axially loaded, cold-formed, built-up C-channels," *Thin-Walled Structures*, vol. 47, no. 2, pp. 190–201, Feb. 2009.
- [21] T. H.-K. Kang, K. Piyawat, and C. Ramseyer, "Design buckling strength curve for built-up cold-formed sections," in *Proc. 6th Int. Symp. on Steel Structures*, Seoul, Korea, Nov. 2011.
- [22] K. Piyawat, C. Ramseyer, and T. H.-K. Kang, "Nonlinear buckling of built-up cold-formed sections," *Int. J. Theoretical and Applied Multiscale Mechanics*, vol. 2, no. 2, pp. 146–164, Nov. 2011.
- [23] K. Piyawat, C. Ramseyer, and T. H.-K. Kang (Dec. 2012). Development of an axial load capacity equation for doubly-symmetric built-up cold-formed sections. *ASCE J. of Structural Engr.* [Online]. Available: <http://ascelibrary.org/doi/abs/10.1061/%28ASCE%29ST.1943-541X.0000780>.
- [24] K. A. Biggs, "Axial load capacity of cold-formed built-up members," MS thesis, The University of Oklahoma, Norman, OK, 2008.
- [25] S. Sukumar and I. L. S. Parameswaran, "Local-, distortional-, and Euler buckling of thin walled built-up open cross-sections under compression," *J. Structural Engr. India (SERC)*, vol. 32, no. 6, pp. 447–454, Feb.-Mar. 2006.
- [26] B. Brueggen and C. Ramseyer, "Cold-formed steel joist member buckling capacity testing," Technical Report, Donald G. Fears Structural Engineering Laboratory, The University of Oklahoma, Norman, OK, 2003.
- [27] T. A. Stone and R. A. LaBoube, "Behavior of cold-formed steel built-up I-sections," *Thin-Walled Structures*, vol. 43, no. 12, pp. 1805–1817, Dec. 2005.
- [28] E. Ellobody and B. Young, "Behavior of cold-formed steel plain angle columns," *ASCE J. Structural Engr.*, vol. 131, no. 3, 457–466, Mar. 2005.
- [29] B. Young and K. J. R. Rasmussen, "Inelastic bifurcation of cold-formed singly symmetric columns," *Thin-Walled Structures*, vol. 36, no. 3, pp. 213–230, Mar. 2000.
- [30] T. V. Galambos, *Guide to Stability Design Criteria for Metal Structures*, 5th ed. Structural Stability Research Council, Wiley, 944,
- [31] Macdonald M, Heiyantuduwa M A et al. (2008). Recent development in the design of cold-formed steel members, *Thin-Walled Structures*, vol 46(7-9), 1047-1053.
- [32] Schafer B W, and Pekoz T (1998). Direct strength prediction of cold-formed steel members using numerical elastic buckling solutions, *Thin-walled Structures*, Shanmugam N E, et al., Ed, Amsterdam: Elsevier.
- [33] Haidarali M R, and Nethercot D A (2012). Local and distortional buckling of cold-formed steel beams with edge-stiffened flanges, *Journal of Constructional Steel Research*, vol 73, 31–42
- [34] Cheng S, Kim B et al. (2013). Lateral torsional buckling of cold-formed channel sections subject to combined compression and bending, *Journal of Constructional Steel Research*, vol 80, 174–180.
- [35] Yu C, and Schafer B W, (2003). Local buckling tests on cold-formed steel beams, *Journal of Structural Engineering*, ASCE, vol 129(12), 1596–1606.
- [36] Yu C, and Schafer B W (2006). Distortional buckling tests on cold-formed steel beams, *Journal of Structural Engineering*, ASCE, vol 132(4), 515–528.
- [37] Lee J and Kim S E (2002). Lateral buckling analysis of thin-walled laminated channel-section beams, *Composite Structures*, vol 56(4), 391–399.
- [38] Seah L K and Khong P W (1990). Lateral-torsional buckling of channel beams, *Journal Construction Steel Research*, vol 17(4), 265–282.